

## Analysis of nailed slope

### Input data

#### Project

Date : 18.9.2006

#### Geometry of structure

Thickness of concrete cover = 0.20 m

No.	Depth Z [m]	Coordinate X [m]
1	0.00	0.00
2	7.00	-2.00

#### Type of nails

No.	Name	Tensile strength $R_t$ [kN]	Pull out resistance $T_p$ [kN/m]	Nail head strength $R_f$ [kN]
1	Typ høebu è. 1	235.62	18.85	22.62

#### Geometry of nails

Overall number of nails - 6

Inclination of nails from horizontal dir. = 10.00 °

Nail	Depth [m]	Depth of joint [m]	Length [m]	Dist. [m]	Type of nail
1	1.00	0.50	3.00	1.00	Typ høebu è. 1
2	2.00	0.50	3.00	1.00	Typ høebu è. 1
3	3.00	0.50	3.00	1.00	Typ høebu è. 1
4	4.00	0.50	3.00	1.00	Typ høebu è. 1
5	5.00	0.50	3.00	1.00	Typ høebu è. 1
6	6.00	1.00	3.00	1.00	Typ høebu è. 1

#### Material of structure

Analysis of concrete structures carried out according to the standard CSN 73 1201 R.

Concrete : B 20

Compressive strength  $R_{bd} = 11.50$  MPa

Tensile strength  $R_{btd} = 0.90$  MPa

Elastic modulus  $E_b = 27000.00$  MPa

Longitudinal steel : 10 216 E

Tensile strength  $R_{sd} = 190.00$  MPa

Compressive strength  $R_{scd} = 190.00$  MPa

Elastic modulus  $E_s = 210000.00$  MPa

#### Soil parameters

##### Clay

Unit weight :  $\gamma = 19,50$  kN/m<sup>3</sup>

Stress-state : effective

Angle of intern. friction :  $\varphi_{ef} = 27,00$  °

Cohesion of soil :  $c_{ef} = 10,00$  kPa

Angle of friction struc.-soil :  $\delta = 15,00$  °

Soil : cohesive




Poisson's ratio :  $\nu = 0,35$

Saturated unit weight :  $\gamma_{\text{sat}} = 19,50 \text{ kN/m}^3$

### Silty Gravel

Unit weight :  $\gamma = 19,50 \text{ kN/m}^3$   
 Stress-state : effective  
 Angle of intern. friction :  $\varphi_{\text{ef}} = 27,00^\circ$   
 Cohesion of soil :  $c_{\text{ef}} = 10,00 \text{ kPa}$   
 Angle of friction struc.-soil :  $\delta = 15,00^\circ$   
 Soil : cohesive  
 Poisson's ratio :  $\nu = 0,35$   
 Saturated unit weight :  $\gamma_{\text{sat}} = 19,50 \text{ kN/m}^3$

### Geological profile and assigned soils

No.	Layer [m]	Assigned soil	Pattern
1	2.00	Clay	
2	3.00	Silty Gravel	
3	-	Clay	

### Terrain profile

Terrain behind construction has the slope 1: 3.73 (slope angle is  $15.00^\circ$ ).  
 Embankment height is 0.54 m, embankment length is 2.00 m.

### Water influence

Ground water table is located below the structure.

### Analysis settings

Active earth pressure calculation - Coulomb (CSN 730037)  
 Passive earth pressure calculation - Caquot-Kerisel (CSN 730037)  
 Standard for concrete structures - CSN 73 1201 R  
 Analysis carried out according to CSN 730037 standard (with reduction of soil input parameters).  
 Internal stability verified according to factor of safety.  
 Factor of safety for plane slip surface  $SF_1 = 1.30$   
 Factor of safety for broken slip surface  $SF_2 = 1.30$

Reduction coeff. of active earth pressure to check for nails bear. capacity  $k_n = 0.85$ .

### Verification No. 1

#### Plane slip surface after optimization :

Slip surface angle =  $33.00^\circ$   
 Origin of slip surface at a depth of = 7.00 m

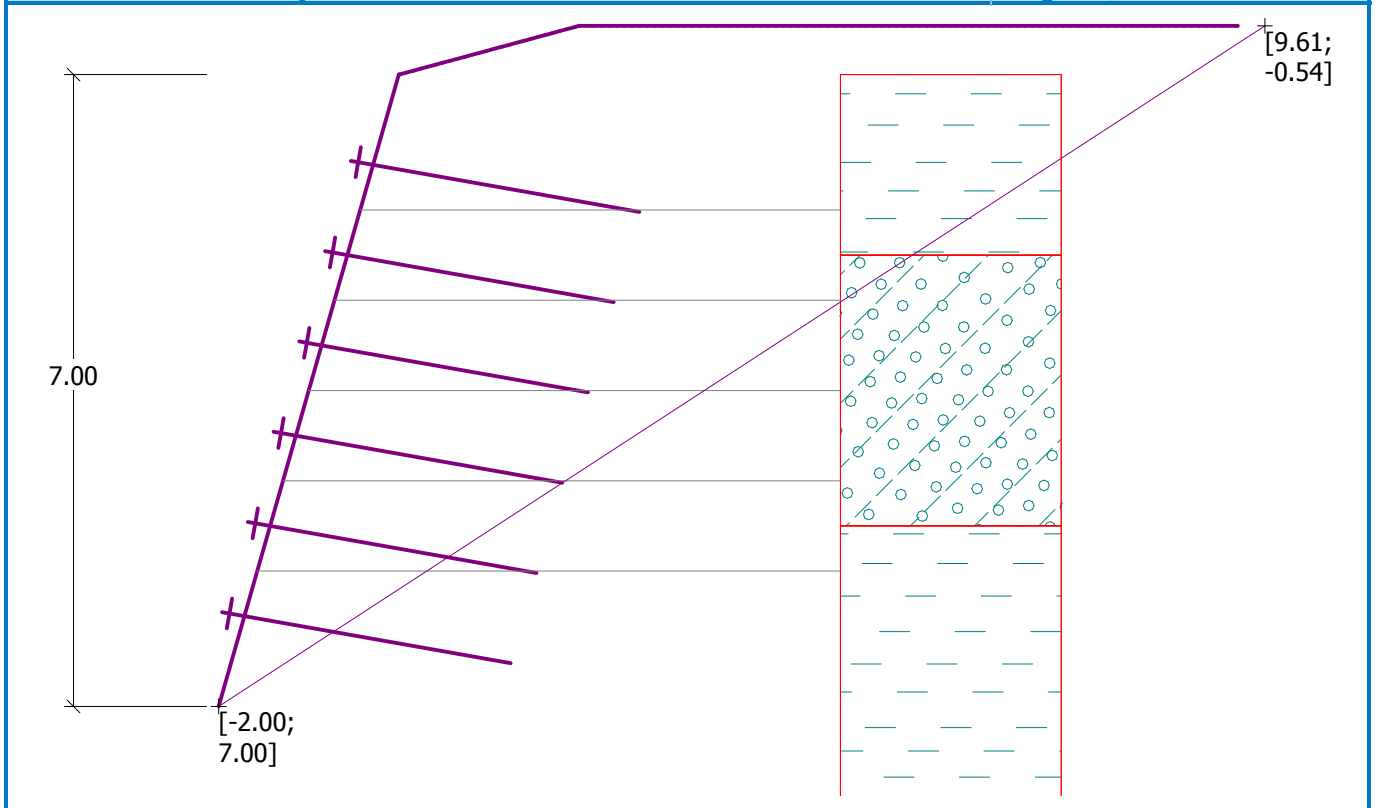
Gravity force = 685.46 kN/m  
 Overall force carried by nails behind slip surf. = 101.88 kN/m  
 Forces on slip surf. driving (grav.force) = 373.33 kN/m  
 Forces on slip surf. driving (pressure) = 0.00 kN/m  
 Forces on slip surf. resist. (soil) = 466.76 kN/m  
 Forces on slip surf. resist. (nails) = 74.51 kN/m

Factor of safety  $F_h/F_m = 1.45 > 1.30$

**Stability of slip surface is SATISFACTORY**

Name : Internal stability

Stage : 1; Verification : 1



## Verification No. 2

### Broken slip surface after optimization :

Slip surface angle =  $22.00^\circ$   
Origin of slip surface at a depth of =  $7.00$  m

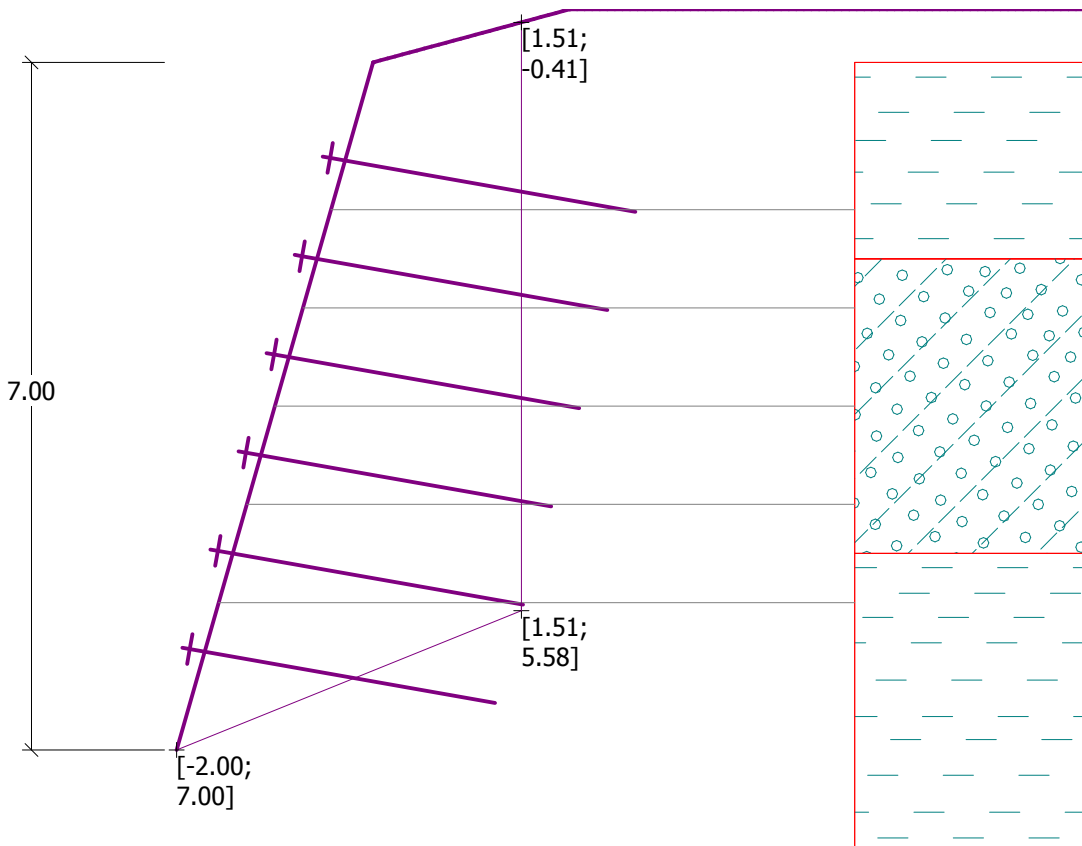
Gravity force	= $299.88$ kN/m
Overall force carried by nails behind slip surf.	= $51.79$ kN/m
Forces on slip surf. driving (grav.force)	= $112.34$ kN/m
Forces on slip surf. driving (pressure)	= $79.24$ kN/m
Forces on slip surf. resist. (soil)	= $193.49$ kN/m
Forces on slip surf. resist. (nails)	= $43.92$ kN/m

Factor of safety  $F_h/F_m = 1.24 < 1.30$

**Stability of slip surface is NOT SATISFACTORY**

Name : Internal stability

Stage : 1; Verification : 2



### Verification No. 3

Horizontal pressure on structure:

Point	Depth [m]	Pressure [kPa]
1	0.00	0.00
2	1.52	0.00
3	2.00	3.21
4	2.57	7.08
5	5.00	19.95
6	7.00	30.55

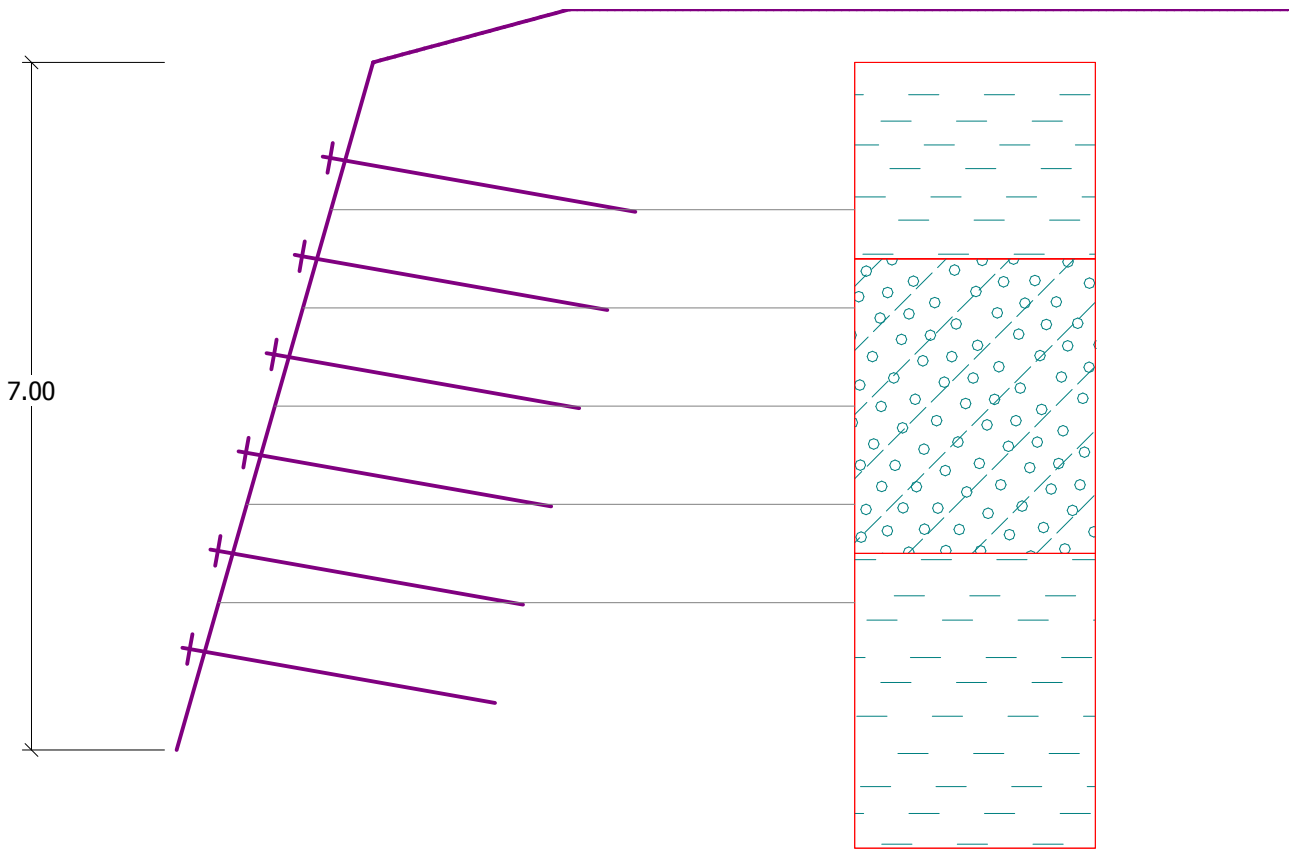
Verification of nails bearing capacity

Nail	Nail bearing capacity [kN]	Nail force [kN]
1	56.55	0.00
2	56.55	2.78
3	56.55	8.06
4	56.55	12.64
5	56.55	17.22
6	56.55	34.40

**Bearing capacity of nails is SATISFACTORY**

Name : Internal stability

Stage : 1; Verification : 3



## Verification No. 1

### Forces acting on construction

Name	$F_{hor}$ [kN/m]	App.Pt. Z [m]	$F_{vert}$ [kN/m]	App.Pt. X [m]	Design coefficient
Gravity force	0.00	-3.45	434.72	2.52	1.000
Active pressure	107.35	-1.47	23.80	3.43	1.000

### Verification of complete wall

#### Check for overturning stability

Resisting moment  $M_{res} = 1058.09$  kNm/m

Overturning moment  $M_{ovr} = 157.52$  kNm/m

**Wall for overturning is SATISFACTORY**

#### Check for slip

Resisting horizontal force  $H_{res} = 208.41$  kN/m

Active horizontal force  $H_{act} = 107.35$  kN/m

**Wall for slip is SATISFACTORY**

#### Forces acting at the centre of footing bottom

Overall moment  $M = -306.68$  kNm/m

Normal force  $N = 458.52$  kN/m

Shear force  $Q = 107.35$  kN/m

**Overall check - WALL is SATISFACTORY**

## Bearing capacity of foundation soil

### Forces acting at the centre of the footing bottom

No.	Moment [kNm/m]	Norm. force [kN/m]	Shear Force [kN/m]	Eccentricity [m]	Stress [kPa]
1	-306.68	458.52	107.35	0.00	147.75

### Bearing capacity of foundation soil check

#### Eccentricity verification

Max. eccentricity of normal force  $e = 0.0$  mm

Maximum allowable eccentricity  $e_{alw} = 1024.1$  mm

**Eccentricity of the normal force is SATISFACTORY**

#### Footing bottom bearing capacity verification

Max. stress at footing bottom  $\sigma = 147.75$  kPa

Bearing capacity of foundation soil  $R_d = 140.00$  kPa

**Bearing capacity of foundation soil is NOT SATISFACTORY**

**Overall verification - bearing capacity of found. soil is NOT SATISFACTORY**

## Dimensioning No. 1

Depth [m]	Horiz.pres. [kPa]	Shear Force [kN/m]	Moment [kNm/m]
0.00	0.00	5.79	0.00
0.33	0.00	5.79	-1.93
0.67	0.00	5.79	-3.86
1.00	0.00	5.79	-5.79
1.00	0.00	-11.57	-5.79
1.50	0.00	-11.57	0.00
1.50	0.00	-11.57	-0.00
1.52	0.00	-11.57	0.28
1.52	0.00	-11.57	0.28
2.00	3.21	-12.34	5.91
2.00	3.21	12.90	5.91
2.50	6.59	10.45	0.00
2.50	6.59	10.45	-0.00
2.57	7.08	9.95	-0.74
2.57	7.08	9.95	-0.74
3.00	9.35	6.44	-4.28
3.00	9.35	-6.00	-4.28
3.50	12.00	-11.34	0.00
4.00	14.65	-18.00	7.28
4.00	14.65	18.44	7.28
4.50	17.30	10.45	0.00
5.00	19.95	1.14	-2.95
5.00	19.95	-0.70	-2.95
5.50	22.60	-11.34	0.00
6.00	25.25	-23.30	8.60

Company Name	Project Name
Project Author	Project Part

Depth [m]	Horiz.pres. [kPa]	Shear Force [kN/m]	Moment [kNm/m]
6.00	25.25	22.11	8.60
6.33	27.02	13.40	2.67
6.67	28.78	4.10	-0.26
6.67	28.78	4.10	-0.26
7.00	30.55	-5.79	0.00

#### Dimensioning of concrete cover in section 6.00 m. (max.moment)

Analysis performed for vertical reinforcement

Reinforcement and dimensions of the cross-section:

Bar diameter = 12.0 mm

Number of bars = 5

Reinforcement cover = 20.0 mm

Cross-section width = 1.00 m

Cross-section depth = 0.20 m

Reinforcement ratio  $\mu_{st} = 0.28 \% > 0.16 \% = \mu_{st,min}$

Position of neutral axis  $x_u = 0.01 m < 0.09 m = x_{u,lim}$

Ultimate moment  $M_u = 16.74 kNm > 8.60 kNm = M_d$

**Cross-section is SATISFACTORY.**